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Reissner型中厚板弯曲理论 中的守恒积分

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摘 要

基于虚功原理导出受弯,中等厚度的,Reissner型板的,与路径无关的 I_R 积分。该积分可作为通常二维I积分的一般情形。在线弹性情形下,还得到了 I_R 与弯曲板应力强度因子K之间的简单关系式。当横向剪切刚度趋于无穷大,则 I_R 变为经典薄板的I积分。

一、引言

自从Rice l^{1} /在平面问题中提出J积分以后,近年来已经有了广泛的应用,特别是在探讨弹塑性断裂理论方面起了较大的作用,在工程界板是常用的一种重要元件,随着断裂力学研究的不断深入,板的断裂分析也日益受到广泛的注意。鉴于J积分对平面 画题富有成效,把类似的守恒积分推广到板的问题中来,也将是一项重要的工作,由于在板的断裂分析中,克希霍夫经典薄板理论有着重大缺陷,因此本文 考虑 Re issner l^{2} 型中厚板弯曲理论。本文从虚功原理出发给出了一个与路线无关的积分 J_R ,在线弹性的情况下,可以求出 J_R 与板的弯曲应力强度因子 K_B 的简单关系。在特殊情况下,此 式 可 以退化为薄板经典理论中的J积分。本文给出的守恒积分在板的弹塑性理论中,可能 会 有重要的作用。在工程断裂的近似计算中,将会发挥较好的效果。

二、JR的积分式和证明

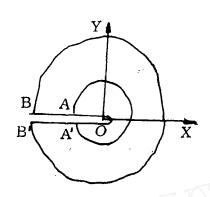
在Re issner型中厚板的理论中, 虚功原理[3]为

$$-\iint_{\Omega} \left[M_{x} \frac{\partial \phi_{x}}{\partial x} + M_{xy} \left(\frac{\partial \phi_{x}}{\partial y} + \frac{\partial \phi_{y}}{\partial x} \right) + M_{y} \frac{\partial \phi_{y}}{\partial y} \right] dx dy +$$

$$+ \iiint_{u} \left[Q_{x} \left(\frac{\partial w}{\partial x} - \phi_{x} \right) + Q_{y} \left(\frac{\partial w}{\partial y} - \phi_{y} \right) \right] dx dy = \iint_{Q} \left(m_{x} \phi_{x} + m_{y} \phi_{y} + Pw \right) dx dy + \int_{Q} \left(Q_{n} \omega - M_{n} \phi_{n} - M_{n} s \phi_{s} \right) dS$$

$$(1)$$

其中内力 M_x 、 M_y 、 M_{xy} 、 Q_x 、 Q_y 应与外载荷保持平衡,即应满足平衡方程,而广义位移 ϕ_x 、 ϕ_y 、w只要连续、一次可导即行。广义位移和广义力之间可以没有任何联系, Ω 是板内的任一区域,C为 Ω 的迈界。



取笛卡尔坐标如图所示,原点在 裂 纹 顶端, x轴平行裂纹而指向物体内,板在边界上受力 而 产生弯曲,即设

$$m_x = m_y = P = 0$$

在裂纹面上:

$$M_y = 0$$
, $M_{xy} = 0$, $Q_y = 0$ (2)

设问题的 7.7 、 O_y ,w, M_x , M_y , M_{xy} , Q_x , Q_y 。

下列广义位移

$$\phi_x^K = \frac{\partial \phi_x}{\partial x}, \quad \phi_y^K = \frac{\partial \phi_y}{\partial y}, \quad w^K = \frac{\partial w}{\partial x}$$
(3)

是一种连续的位移, 所以根据公式(1) 有

$$\iint_{\Omega} \left[-M_{x} \frac{\partial \psi_{x}^{K}}{\partial x} - M_{xy} \left(\frac{\partial \psi_{y}^{K}}{\partial y} + \frac{\partial \psi_{y}^{K}}{\partial x} \right) - M_{y} \frac{\partial \psi_{y}^{K}}{\partial y} + Q_{x} \left(\frac{\partial w^{K}}{\partial x} - \psi_{x}^{K} \right) \right] + Q_{x} \left(\frac{\partial w^{K}}{\partial y} - \psi_{y}^{K} \right) dS \qquad (4)$$

这里 Ω 是板内的任一区域,C为 Ω 的边界,现在取C为图中所示的A' B' BAA',因为A' B' 与BA为不受力的自由边,所以(4)式的右端化为

$$\frac{A_{1}}{m} = \int_{B_{I}}^{B} (Q_{n}w^{K} - M_{n}\phi_{n}^{K} - M_{n}S\phi_{S}^{K})dS + \int_{A_{I}}^{A_{I}} (Q_{n}w^{K} - M_{n}\phi_{n}^{K} - M_{n}S\phi_{S}^{K})dS$$

$$= \int_{B_{I}}^{B} (Q_{n}w^{K} - M_{n}\phi_{n}^{K} - M_{n}\phi_{S}^{K})dS - \int_{A_{I}}^{A_{I}} (Q_{n}w^{K} - M_{n}\phi_{n}^{K} - M_{n}S\phi_{S}^{K})dS \qquad (5)$$

又根据(3)式, 行

$$-M_{x}\frac{\partial \phi_{x}^{K}}{\partial x}-M_{x,y}\left(\frac{\partial \phi_{x}^{K}}{\partial y}+\frac{\partial \phi_{y}^{K}}{\partial x}\right)-M_{x}\frac{\partial \phi_{y}^{K}}{\partial y}+Q_{x}\left(\frac{\partial w^{K}}{\partial x}-\phi_{x}^{K}\right)+Q_{y}\left(\frac{\partial w^{K}}{\partial y}-\phi_{y}^{K}\right)$$

$$= \frac{\partial U'}{\partial K_x} \cdot \frac{\partial K_x}{\partial x} + \frac{\partial U'}{\partial K_{xy}} \cdot \frac{\partial K_{xy}}{\partial x} + \frac{\partial U'}{\partial K_y} \cdot \frac{\partial K_y}{\partial x} + \frac{\partial U''}{\partial x_x} \cdot \frac{\partial Y_x}{\partial x_y} + \frac{\partial U''}{\partial Y_y} \cdot \frac{\partial Y_y}{\partial y}$$

$$= \frac{\partial}{\partial x} \left(U' + U'' \right) = \frac{\partial U}{\partial x}$$
(6)

式中U为应变能密度,U'为弯曲应变能密度,U''为剪切应变能密度,广义应变 K_* , K_* , K_* , Y_* , Y_* , Y_* , Y_*

$$K_{x} = -\frac{\partial \psi_{x}}{\partial x}, \quad K_{y} = -\frac{\partial \psi_{y}}{\partial y}, \quad K_{xy} = -\frac{1}{2} \left(\frac{\partial \psi_{x}}{\partial y} + \frac{\partial \psi_{y}}{\partial x} \right)$$

$$\gamma_{x} = \frac{\partial w}{\partial x} - \psi_{x}, \quad \gamma_{x} = \frac{\partial w}{\partial y} - \psi_{y}$$

$$(7)$$

因此, (4) 式的左端为

左端 =
$$\iint_{\mathcal{O}} \frac{\partial U}{\partial x} dx dy = \int_{C} lU dS = \int_{B}^{B} lU dS - \int_{A}^{A} lU dS$$
 (8)

从(6),(8)两式得到

$$\int_{B'}^{B} lUdS - \int_{B'}^{B} (Q_n w^K - M_n \phi_n^K - M_n S \phi_s^K) dS$$

$$= \int_{A'}^{A} lUdS - \int_{A'}^{A} (Q_n w^K - M_n \phi_n^K - M_n s \phi_s^K) dS$$
 (9)

此式表示上列积分与路径无关,把它记为JR,则有

$$J_R = \int_C (lU - Q_n w^K + M_n \phi_n^K + M_{nS} \phi_s^K) dS$$

$$I_{R} = \int_{C} (1U - Q_{n} \frac{\partial w}{\partial x} + M_{n} \frac{\partial \phi_{n}}{\partial x} + M_{nS} \frac{\partial \phi_{S}}{\partial x}) dS$$
 (10)

与路径无关,只要C为由裂纹下面绕端点到正面一圈。

三、JR与弯曲应力強度因子KB关系

在Re is sner型理论中, 裂纹顶端内力和位移的首项为

$$M_{x} = \frac{h^{2}}{6} \left[\frac{K_{BI}}{\sqrt{2\pi\gamma}} \cos\frac{\theta}{2} \left(1 - \sin\frac{\theta}{2} \sin\frac{3\theta}{2} \right) - \frac{K_{BI}}{\sqrt{2\pi\gamma}} \sin\frac{\theta}{2} \left(2 + \cos\frac{\theta}{2} \cos\frac{3\theta}{2} \right) \right]$$

$$M_{y} = \frac{h^{2}}{6} \left[\frac{K_{BI}}{\sqrt{2\pi\gamma}} \cos\frac{\theta}{2} \left(1 + \sin\frac{\theta}{2} \sin\frac{3\theta}{2} \right) + \frac{K_{BI}}{\sqrt{2\pi\gamma}} \sin\frac{\theta}{2} \cos\frac{\theta}{2} \cos\frac{3\theta}{2} \right]$$

$$M_{xy} = \frac{h^{2}}{6} \left[\frac{K_{BI}}{\sqrt{2\pi\gamma}} \sin\frac{\theta}{2} \cos\frac{\theta}{2} \cos\frac{3\theta}{2} + \frac{K_{BI}}{\sqrt{2\pi\gamma}} \cos\frac{\theta}{2} \left(1 - \sin\frac{\theta}{2} \sin\frac{3\theta}{2} \right) \right]$$

$$Q_{\pi} = -\frac{2h}{3} \cdot \frac{K_{BII}}{\sqrt{2\pi\gamma}} \sin \frac{\theta}{2}$$

(11)

$$Q_y = \frac{2h}{3} \cdot \frac{K_{BII}}{\sqrt{2\pi\gamma}} \cos \frac{\theta}{2}$$

$$\psi_x = -\frac{K_{BI}}{2Gh} \sqrt{\frac{\gamma}{2\pi}} \left[(2K - 1) \cos \frac{\theta}{2} - \cos \frac{3\theta}{2} \right] - \frac{K_{BI}}{2Gh} \sqrt{\frac{\gamma}{2\pi}} \left[(2K + 3) \sin \frac{\theta}{2} + \sin \frac{3\theta}{2} \right]$$

$$\phi_{y} = -\frac{K_{B1}}{2Gh} \sqrt{\frac{\gamma}{2\pi}} \left[(2K+1)\sin\frac{\theta}{2} - \sin\frac{3\theta}{2} \right] - \frac{K_{B1}}{2Gh} \sqrt{\frac{\gamma}{2\pi}} \left[-(2K-3)\cos\frac{\theta}{2} - \cos\frac{3\theta}{2} \right]$$

$$-\cos\frac{3\theta}{2}$$

$$w = \frac{8K_{B1}}{5G} \sqrt{\frac{\gamma}{2\pi}} \sin\frac{\theta}{2}$$

当(10)式中的C取得十分靠近裂纹顶端时,内力和位移便足够精 确 地 用 (11) 式 来 表示。

将(11)式代入(10)式,算出积分,便可把 J_R 与 K_{BI} , K_{BI} , K_{BI} 联系 起 来,经 一 些运算后,可得

$$\int_{C} IUdS = \frac{h(1-\nu)}{12E} (K_{BI}^{2} - K_{BI}^{2})$$

$$\int_{C} (-Q_{n} \frac{\partial w}{\partial x} + M_{n} \frac{\partial \psi_{n}}{\partial x} + M_{n} \frac{\partial \psi_{S}}{\partial x}) dS = \frac{hK_{BI}^{2}}{12E} (3+\nu) + \frac{hK_{BI}^{2}}{12E} (5-\nu) + \frac{4h}{15G} K_{BI}^{2}$$

p[I]

$$J_{R} = \frac{h}{3E} (K_{B1}^{2} + K_{B1}^{2}) + \frac{4h}{15G} K_{B1}^{2}$$
 (12)

当考虑经典板的情况,即

$$\phi_n = \frac{\partial w}{\partial n} , \quad \phi_S = \frac{\partial w}{\partial S}$$
(13)

这时,

$$J = \int_{C} \left[1U + \left(-Q_{n} \frac{\partial w}{\partial x} - M_{n} \frac{\partial \phi_{n}}{\partial x} + M_{n} \frac{\partial \phi_{s}}{\partial x} \right) \right] dS$$

$$= \int_{C} \left[1U + \left(-Q_{n} \frac{\partial w}{\partial x} - M_{n} \frac{\partial^{2} w}{\partial n \partial x} + M_{n} \frac{\partial^{2} w}{\partial s \partial x} \right) \right] dS$$

$$= \int_{C} \left[1U + \left(-Q_{n} \frac{\partial w}{\partial x} - M_{n} \frac{\partial^{2} w}{\partial n \partial x} - \frac{\partial M_{n} s}{\partial s} \cdot \frac{\partial w}{\partial x} \right) \right] dS$$

$$= \int_{C} \left\{ 1U + \left[-\left(Q_{n} + \frac{\partial M_{n} s}{\partial s} \right) \frac{\partial w}{\partial x} + M_{n} \frac{\partial^{2} w}{\partial n \partial x} \right\} dS$$

$$(14)$$

(14)式为经典薄板理论的守恒积分形式。

参考 文献

- [1] Rice, J.R., A path independent integral and the approximate analysis of strain concentration by notches and crack, J. Appl. Mech. 1968, 379-386.
- [2] Reissner, E., On bending of elastic plates, Quarterly of Applied Mathematics, 5 1947 55-68.
- [3] 胡海昌,弹性力学的变分原理及其应用,科学出版社, 1981。

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A Conservative Integral for Reissner

Type Moderate Thick Plate in Bending

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ABSTRACT

Based on the virtual work principle, a path-independent integral I_R for the Reissner type moderate thick plate in bending is obtained. It can be regarded as a generalization of the conventional 2D I integral. In the linear elastic case a simple relationship between I_R and the bending stress intensity factor K is also obtained. When transverse shear stiffness tend to infinity, I_R is reduced to the I integral for the classical thin plate.